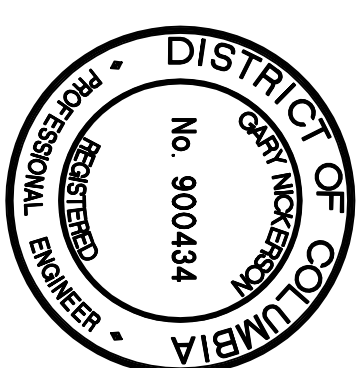




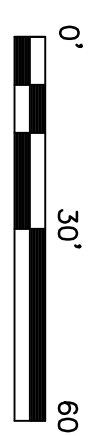
WEST
PERIMETER
ROAD
GEORGETOWN
UNIVERSITY
WASHINGTON, DC

SEAL



KEY PLAN

SCALE

[illegible]

REVISION

DRAWN BY

100

DATE _____

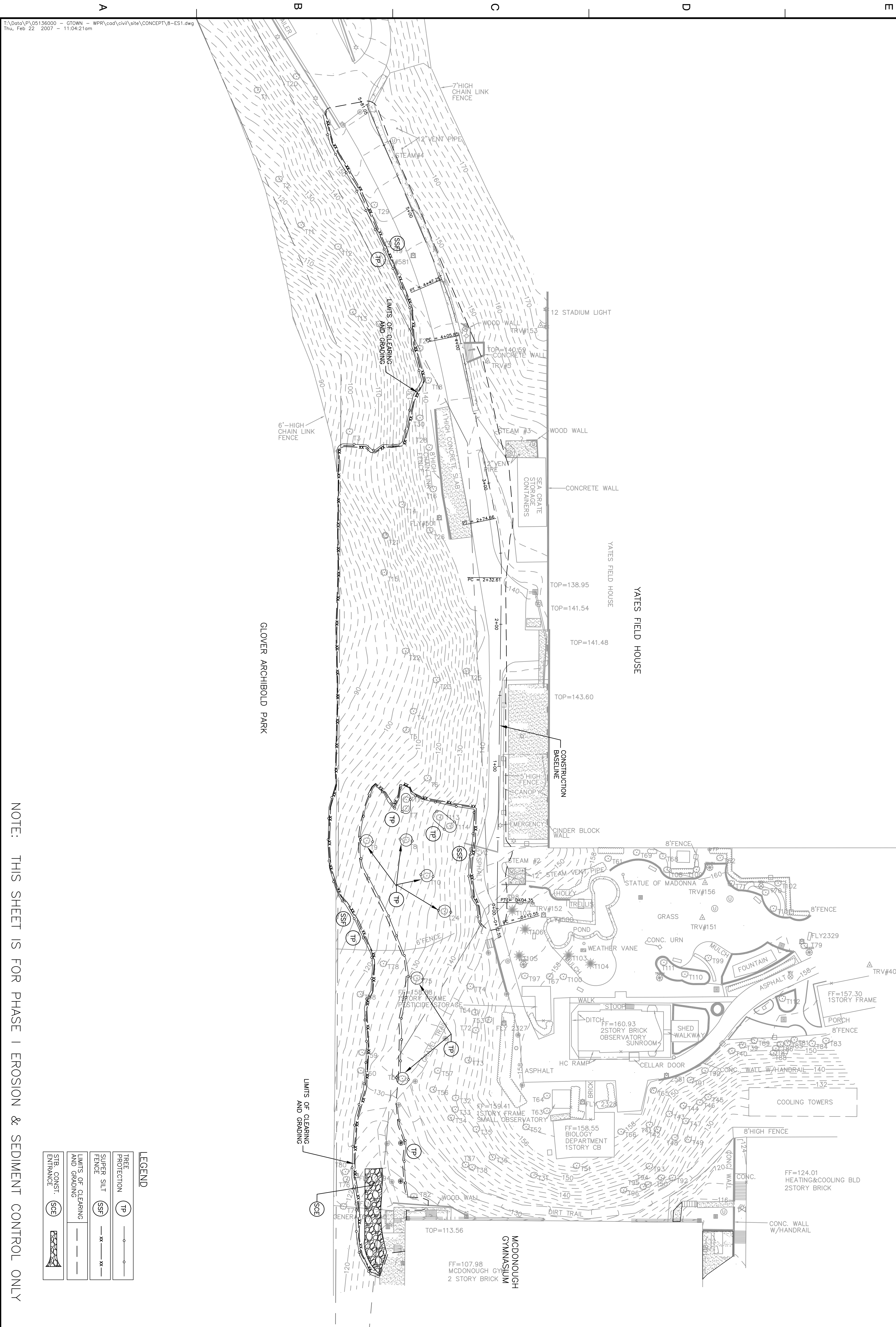
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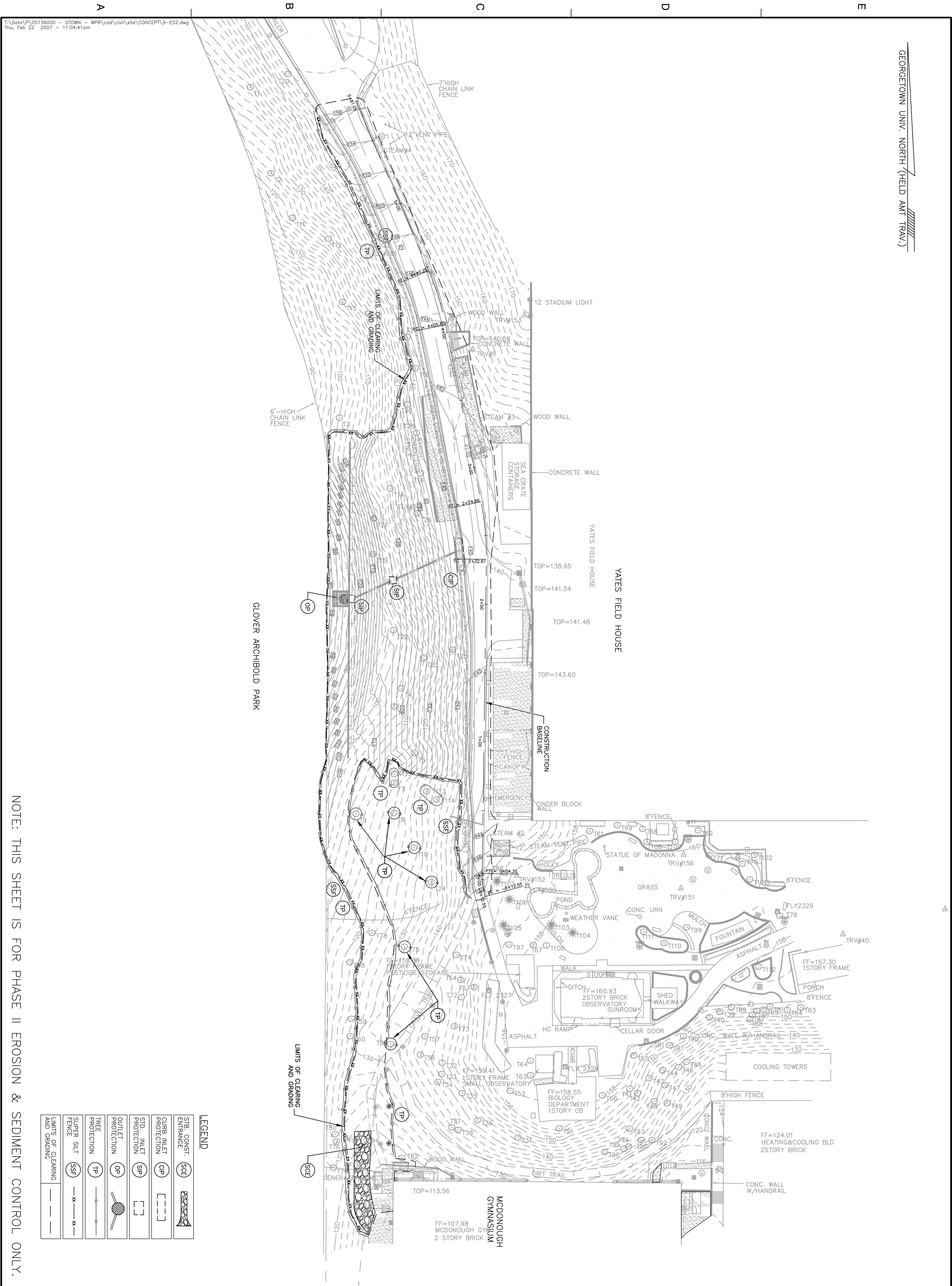
EROSION & SEDIMENT CONTROL PHASE I

PROJECT NO. 0513600C

SHEET NO

8 OF 18





NOTE: THIS SHEET IS FOR PHASE II EROSION & SEDIMENT CONTROL ONLY.

Dewberry & Davis, LLC
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SUITE 400
WASHINGTON, DC 20014
PHONE: 703.348.0100
FAX: 703.348.0118

WEST PERIMETER ROAD
GEORGETOWN UNIVERSITY
WASHINGTON, DC

SCALE
0' 30' 60'

KEY PLAN

No.	DATE	BY	Description

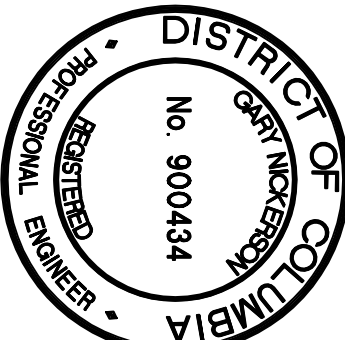
REVISIONS
DRAWN BY A. SPRUCH
APPROVED BY T. CULLETON
CHECKED BY T. CULLETON
DATE JANUARY 2007

PROJECT NO. 05136000

EROSION & SEDIMENT CONTROL PHASE II

SHEET NO. 9

9 OF 18



KEY PLAN

SCALE

No.	DATE	BY	Description

REVISIONS

DRAWN BY A. SPRUCH

APPROVED BY T. CULLETON

CHECKED BY T. CULLETON

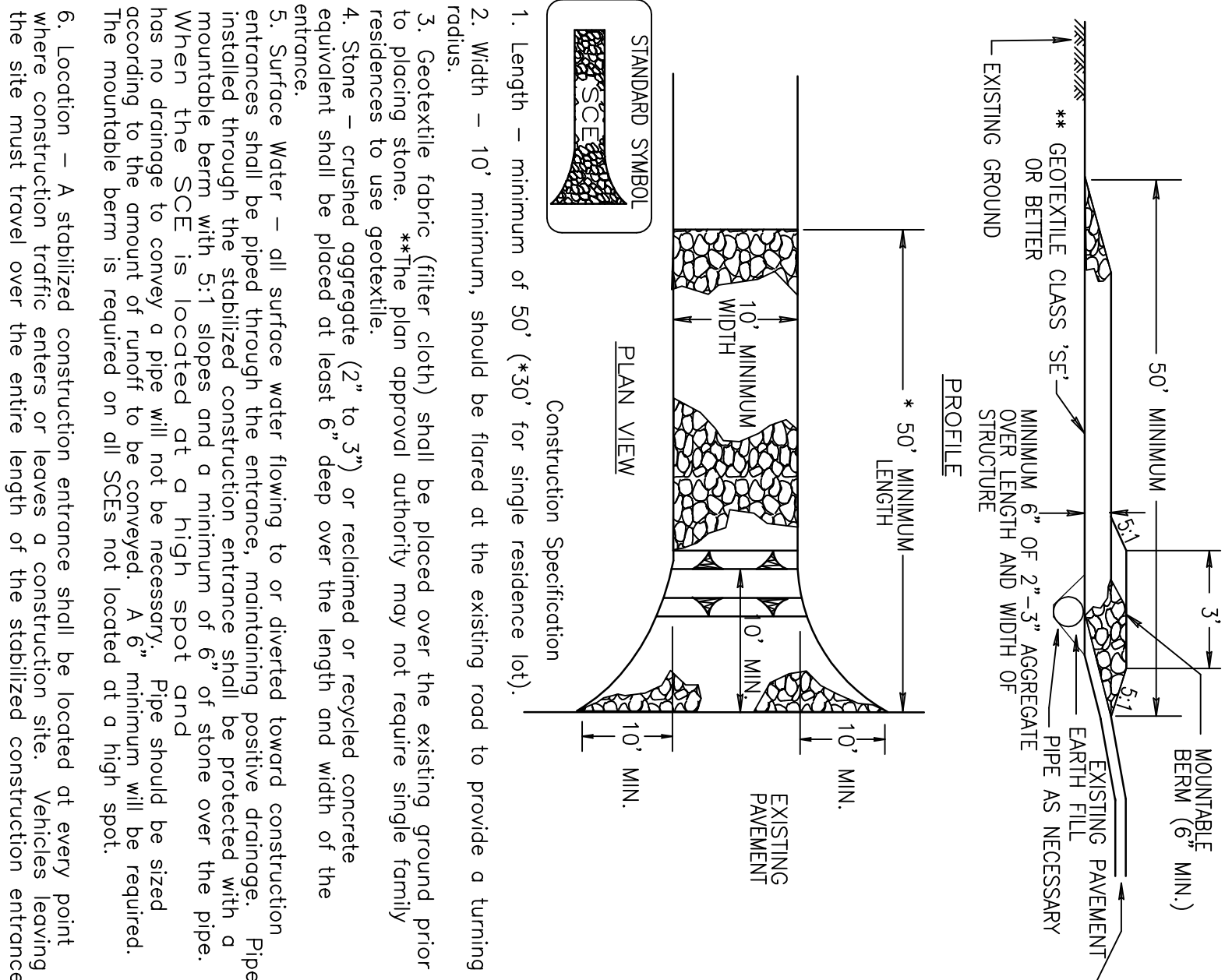
DATE JANUARY 2007

EROSION & SEDIMENT CONTROL DETAILS

PROJECT NO. 05136000

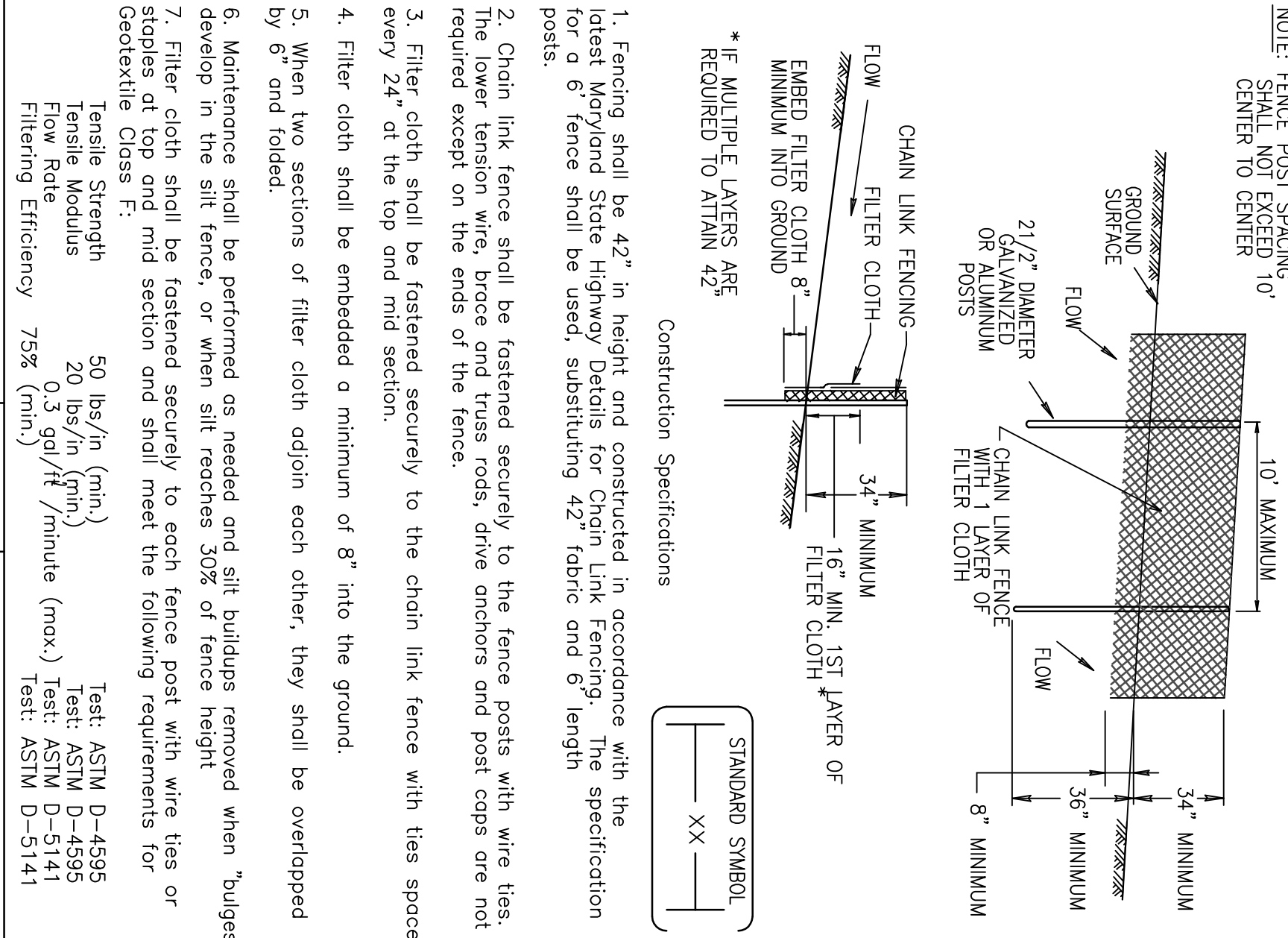
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DETAIL 1 – STABILIZED CONSTRUCTION ENTRANCE



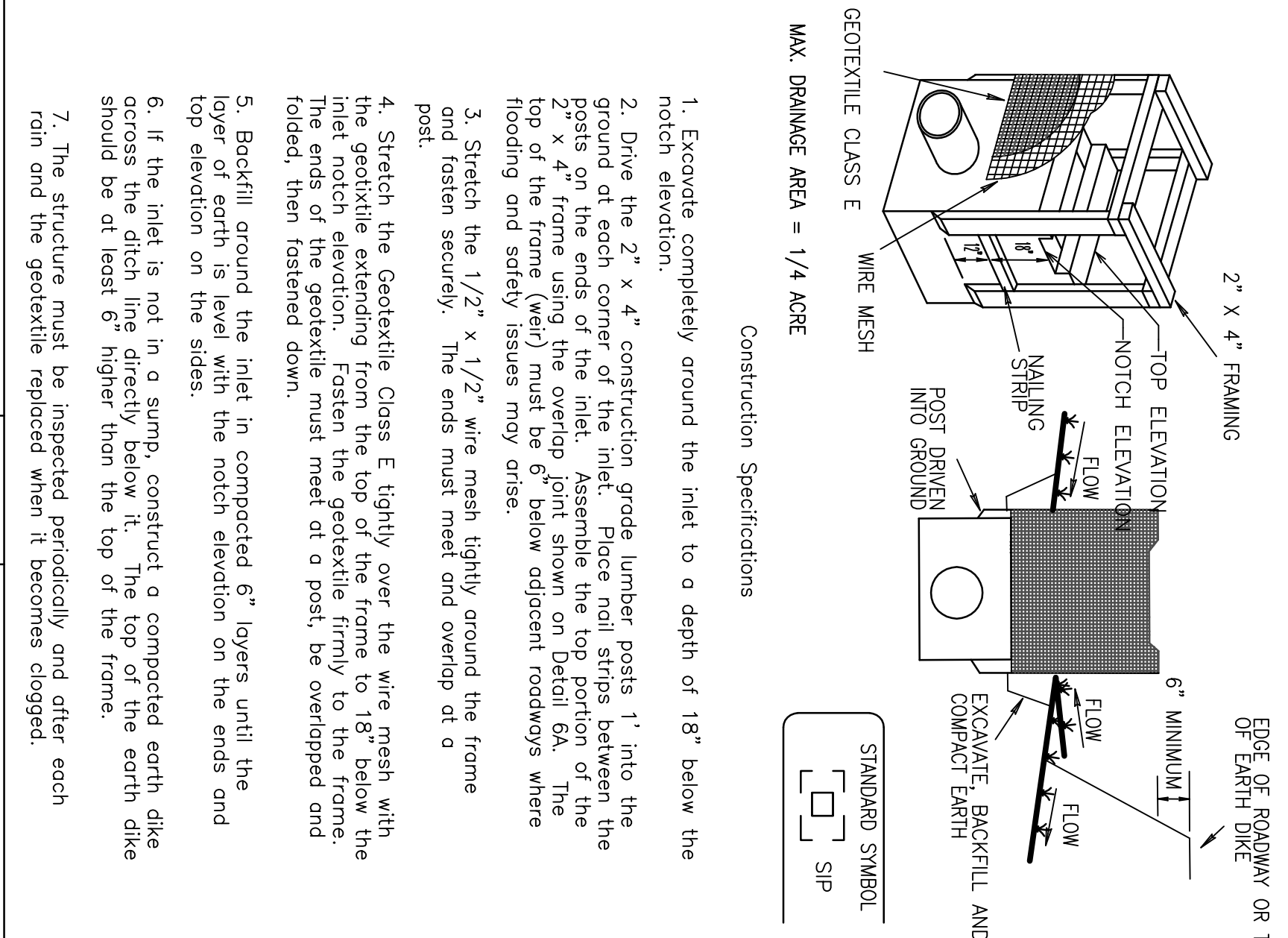
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DETAIL 5 – SUPER SILT FENCE

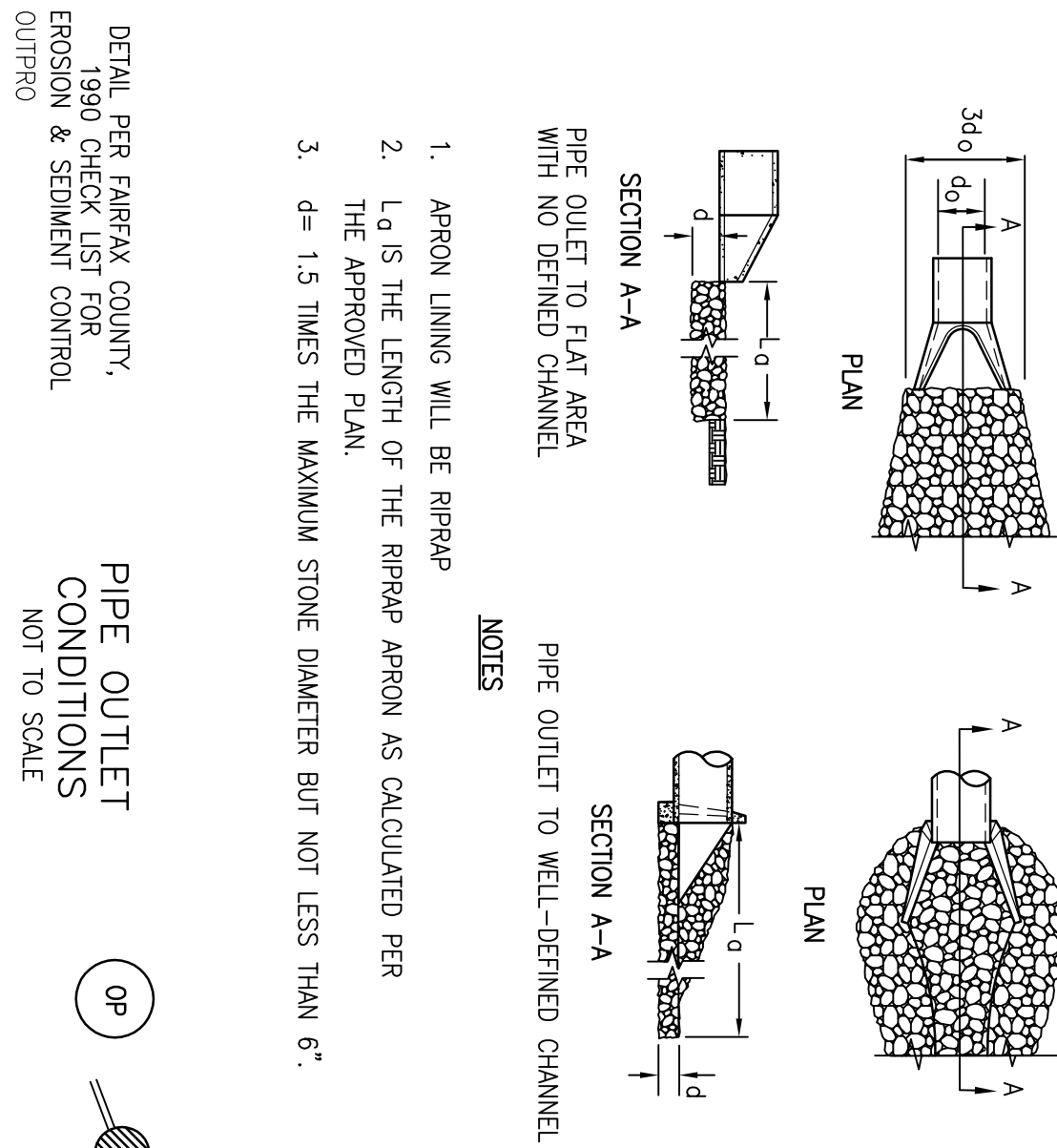


3

DETAIL 6A – STANDARD INLET PROTECTION

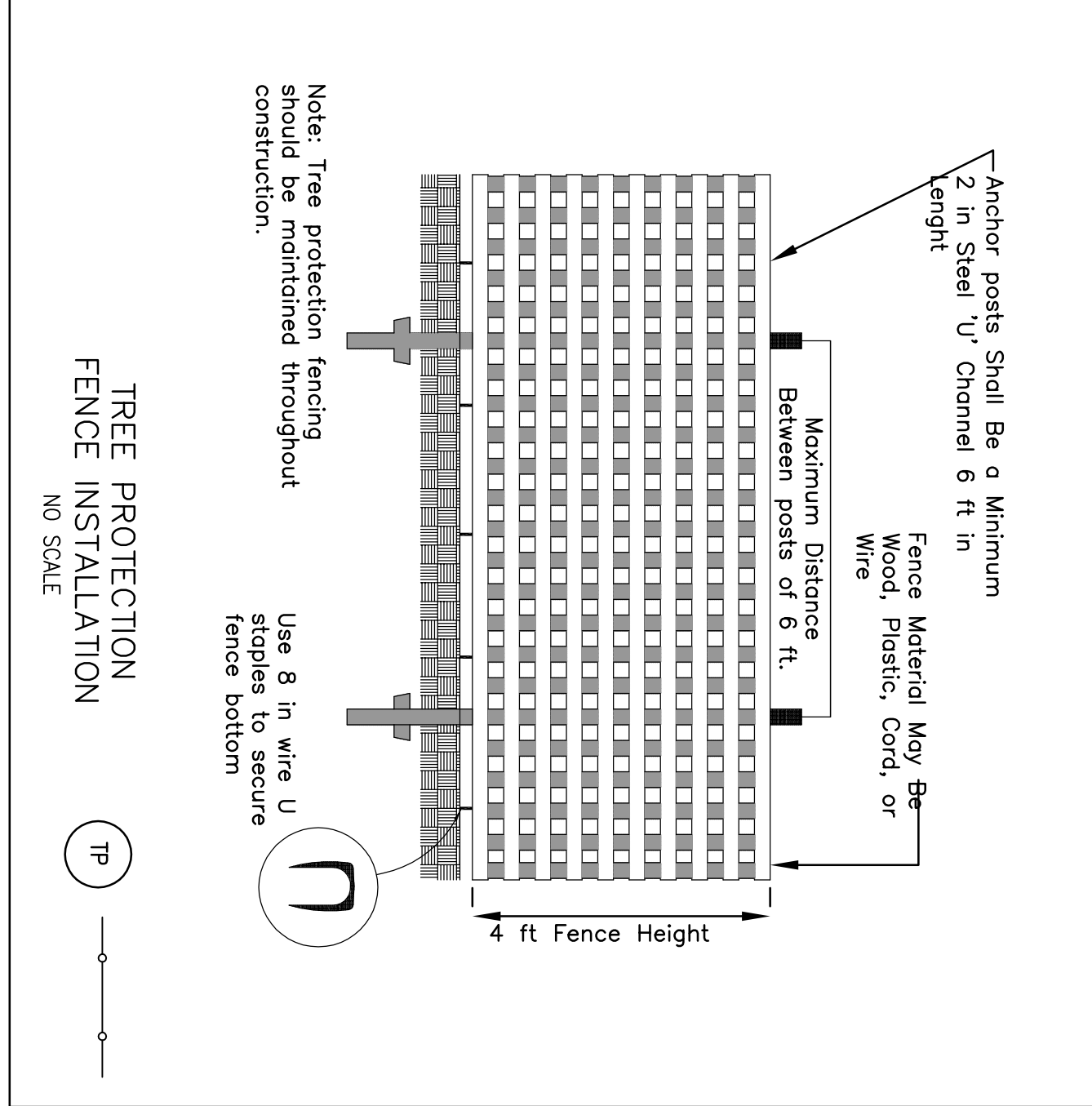
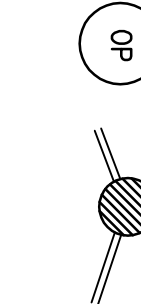


4



DETAIL PER FAIRFAX COUNTY, 1890 CHECK LIST FOR EROSION & SEDIMENT CONTROL OUTFALL

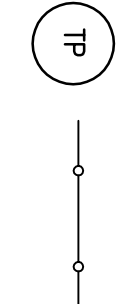
PIPE OUTLET CONDITIONS NOT TO SCALE



Note: Tree protection fencing should be maintained throughout construction.

Use 8 in wire U staples to secure fence bottom

TREE PROTECTION FENCE INSTALLATION NO SCALE



5

U.S. DEPARTMENT OF AGRICULTURE NATURAL RESOURCE CONSERVATION SERVICE	PAGE A-1 - 4	WATERSHED PROTECTION DIVISION DISTRICT OF COLUMBIA DEPARTMENT OF HEALTH	U.S. DEPARTMENT OF AGRICULTURE NATURAL RESOURCE CONSERVATION SERVICE	PAGE B-6 - 4	WATERSHED PROTECTION DIVISION DISTRICT OF COLUMBIA DEPARTMENT OF HEALTH	U.S. DEPARTMENT OF AGRICULTURE NATURAL RESOURCE CONSERVATION SERVICE	PAGE B-7 - 7	WATERSHED PROTECTION DIVISION DISTRICT OF COLUMBIA DEPARTMENT OF HEALTH
CONCEPT 10-ES_DET1.dwg								

U.S. DEPARTMENT OF AGRICULTURE NATURAL RESOURCE CONSERVATION SERVICE	PAGE A-1 - 3	WATERSHED PROTECTION DIVISION DISTRICT OF COLUMBIA DEPARTMENT OF HEALTH	U.S. DEPARTMENT OF AGRICULTURE NATURAL RESOURCE CONSERVATION SERVICE	PAGE B-6 - 3	WATERSHED PROTECTION DIVISION DISTRICT OF COLUMBIA DEPARTMENT OF HEALTH

STABILIZED CONSTRUCTION ENTRANCE

SUPER SILT FENCE

DETAIL 6C – CURB INLET PROTECTION (CDD OR CDS INLETS)

Construction Specification

1. Length – minimum of 50' (30' for single residence lot).
2. Width – 10' minimum, should be flared at the existing road to provide a turning radius.
3. Geotextile fabric (filter cloth) shall be placed over the existing ground prior to placing stone. The plan approval authority may not require single family residences to use geotextile.
4. Stone – crushed aggregate (2" to 3"), or reclaimed or recycled concrete equivalent shall be placed at least 6" deep over the length and width of the entrance.
5. Surface Water – all surface water flowing to or diverted toward construction entrances shall be piped through the entrance, maintaining positive drainage. Pipe installed through the stabilized construction entrance shall be protected with a mountable berm with 5:1 slopes and a minimum of 6" of stone over the pipe. Pipe has to be sized according to the drainage. When the SCE is located at a high spot and the pipe is located at a low spot, the pipe shall be installed above the ground according to the amount of runoff to be conveyed. A 6" minimum will be required.
6. Location – A stabilized construction entrance shall be located at every point where construction traffic enters or leaves a construction site. Vehicles leaving the site must travel over the entire length of the stabilized construction entrance.

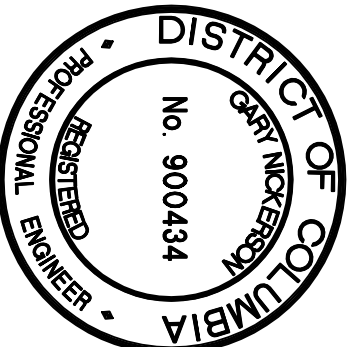
Design Criteria

Slope	Slope Steepness	Slope Length (maximum)	Silt Fence Length (maximum)
0 – 10%	0 – 10:1	Unlimited	Unlimited
10 – 20%	10:1 – 5:1	200 feet	1,500 feet
20 – 33%	5:1 – 3:1	100 feet	1,000 feet
33 – 50%	3:1 – 2:1	100 feet	500 feet
50% +	2:1 +	50 feet	250 feet

Construction Specifications

1. Attach a continuous piece of wire mesh (30" minimum width by throat length plus 4") to the 2" x 4" weir (measuring throat length plus 2") as shown on the standard drawing.
2. Place a continuous piece of Geotextile Class E the same dimensions as the wire mesh over the wire mesh and securely attach it to the 2" x 4" weir.
3. Securely nail the 2" x 4" weir to a 9" long vertical spacer to be located between the weir and the inlet face (max. 4' apart).
4. Place the assembly against the inlet throat and nail (minimum 2" lengths of 2" x 4" to the top of the weir at spacer locations). These 2" x 4" anchors shall extend across the inlet top and be held in place by sandbags or alternate weight.
5. The assembly shall be placed so that the end spacers are a minimum 1' beyond both ends of the throat opening.
6. Form the 1/2" x 1/2" wire mesh and the geotextile fabric to the concrete gutter and stone over the face of the curb on both sides of the inlet. Place clean 3/4" x 1 1/2" entering the inlet under or around the geotextile.
7. This type of protection must be inspected frequently and the filter cloth and stone replaced when clogged with sediment.
8. Assume that storm flow does not bypass the inlet by installing a temporary earth or asphalt dike to direct the flow to the inlet.

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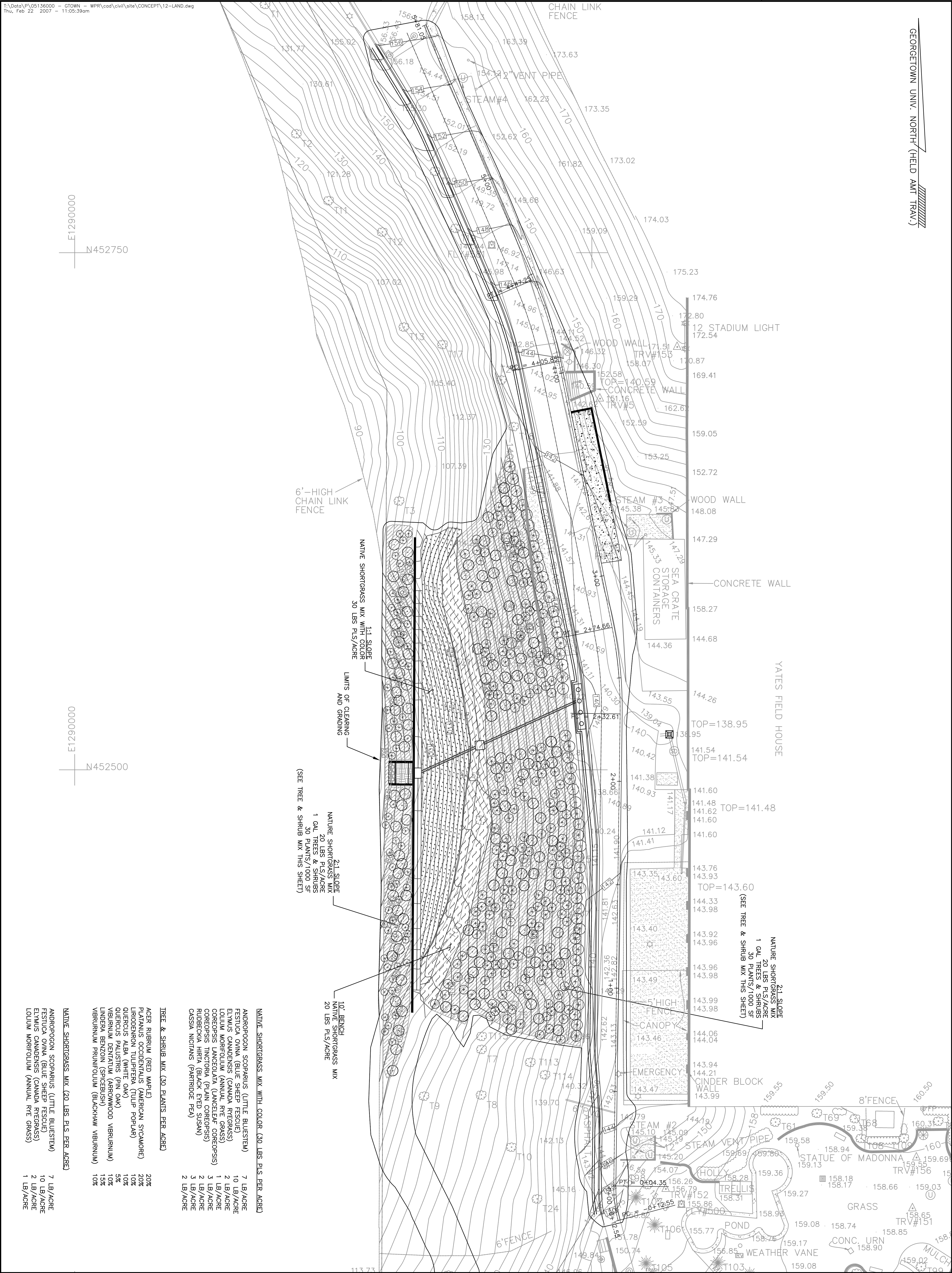


No.	DATE	BY	Description

REVISIONS	
DRAWN BY	A. SPRUCH
APPROVED BY	T. CULLETON
CHECKED BY	T. CULLETON
DATE	JANUARY 2007

LANDSCAPE PLAN

PROJECT NO. 05136000



SOURWATER MANAGEMENT NARRATIVE

THE PROPOSED MAINTENANCE PROJECT IS REQUIRED TO STABILIZE FAILING SLOPES JUST WEST OF TATES FIELD HOUSE, BETWEEN THE WEST PERIMETER ROAD AND ADJACENT GLOVER ARCHBOLD PARK. THE PROPOSED PROJECT WILL INVOLVE THE CONSTRUCTION OF A 15" DIA. STORMWATER INLET AND A 15" DIA. STORMWATER PIPE TO BE INSTALLED IN THE AREA OF POORLY COMPACTED FILL ADJACENT TO THE EXISTING FIELD HOUSE. AT THE TOE OF THE FILL, THE DRAINAGE SWALE ON THE PARK PROPERTY COVERS STORM DRAINAGE WITHOUT VISIBLE SIGNS OF EROSION. THE PROPOSED LEVEL SPREADER AT THE TOE OF THE PROPOSED SLOPE STABILIZATION WILL LIMIT EXISTING VELOCITY AND GUARD AGAINST OUTFALL EROSION.

STORM SEWER COMPUTATIONS

FROM STR	TO STR	AREA	TOTAL RUNOFF	CA	ACCUM TO	I IN/HR	Q INQ	Q TOTAL	DIA IN	SLOPE FT/FT	LENGTH FT	N	Q CAP	Q CAP	DROP FT	UPPER INVERT	LOWER INVERT
#	#	ac	ac	C	CA	CA	MIN	CFS	CFS	CFS	CFS	CFS	CFS	CFS	CFS	FT	FT
1A	1B	0.99	0.99	0.67	0.66	0.66	5.00	7.56	5.01	5.01	15.00	0.3580	49.00	0.013	41.57	31.49	17.54
1B	1C	0.12	1.11	0.35	0.04	0.70	5.00	7.56	0.30	5.31	15.00	0.5470	30.42	0.013	51.39	38.93	15.92
																106.50	90.58

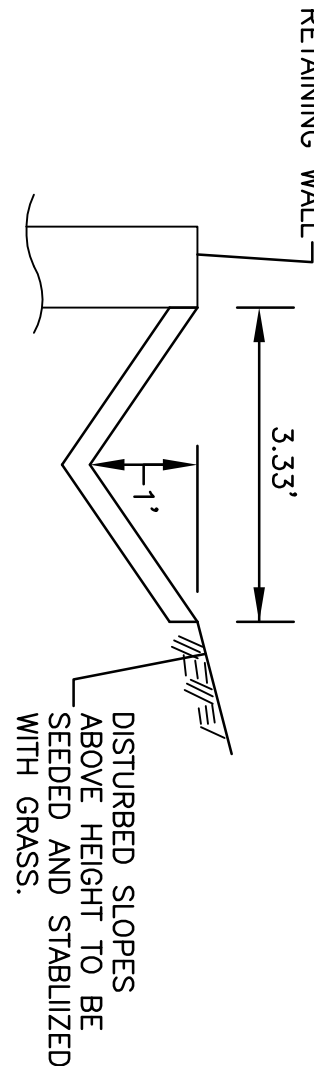
STORM INLET COMPUTATIONS

PROJECT DESCRIPTION: 15 YEAR STORM				PROJECT DESCRIPTION: 15 YEAR STORM				PROJECT DESCRIPTION: 15 YEAR STORM			
WORKSHEET TYPE: CURB INLET 1A CURE INLET IN SAG SPREAD				WORKSHEET TYPE: GRATE INLET - 1B GRATE INLET ON GRADE				WORKSHEET TYPE: GRATE INLET - 1C GRATE INLET IN SAG SPREAD			
SOLVE FOR				SOLVE FOR				SOLVE FOR			
INPUT DATA:				INPUT DATA:				INPUT DATA:			
DISCHARGE	5.01 CFS	GUTTER WIDTH	1.00 FT	DISCHARGE	0.30 CFS	GUTTER WIDTH	1.16 FT	DISCHARGE	0.92 CFS	GUTTER WIDTH	0.90 FT
GUTTER CROSS SLOPE	0.063 FT/FT	GUTTER CROSS SLOPE	0.02 FT/FT	GUTTER CROSS SLOPE	0.02 FT/FT	GUTTER CROSS SLOPE	0.02 FT/FT	GUTTER CROSS SLOPE	0.00 FT/FT	GUTTER CROSS SLOPE	0.144 FT/FT
ROAD CROSS SLOPE	0.02 FT/FT	DITCH CROSS SLOPE	0.05 FT/FT	DITCH CROSS SLOPE	0.05 FT/FT	DITCH CROSS SLOPE	0.05 FT/FT	DITCH CROSS SLOPE	0.144 FT/FT	DITCH CROSS SLOPE	0.144 FT/FT
CURB OPENING LENGTH	16.00 FT	GRATE WIDTH	3.42 FT	GRATE WIDTH	3.42 FT	GRATE WIDTH	3.42 FT	GRATE WIDTH	3.42 FT	GRATE WIDTH	3.42 FT
OPENING HEIGHT	0.50 FT	GRATE LENGTH	5.30 MM(P-1-7/8")	GRATE LENGTH	5.30 MM(P-1-7/8")	GRATE LENGTH	5.30 MM(P-1-7/8")	GRATE LENGTH	5.30 MM(P-1-7/8")	GRATE LENGTH	5.30 MM(P-1-7/8")
CURB DEPRESSION	1.00 IN	LOCAL DEPRESSION	0.06 IN	LOCAL DEPRESSION	0.06 IN	LOCAL DEPRESSION	0.06 IN	LOCAL DEPRESSION	0.06 IN	LOCAL DEPRESSION	0.06 IN
LOCAL DEPRESSION WIDTH	100 FT	CLOSING	50.0%	CLOSING	50.0%	CLOSING	50.0%	CLOSING	50.0%	CLOSING	50.0%
RESULTS:				RESULTS:				RESULTS:			
SPREAD	12.32 FT	THROAT INCLINE ANGLE	90.00 DEGREES	SPREAD	5.10 FT	DEPTH	0.10 FT	SPREAD	2.58 FT	DEPTH	0.37 FT
DEPTH	0.31 FT	EFFICIENCY	96.19%	DEPTH	0.10 FT	EFFICIENCY	96.19%	DEPTH	0.37 FT	EFFICIENCY	96.19%
OPEN DEPRESSION	0.06 IN	OPEN DEPRESSION	0.06 IN	OPEN DEPRESSION	0.06 IN	OPEN DEPRESSION	0.06 IN	OPEN DEPRESSION	0.06 IN	OPEN DEPRESSION	0.06 IN
TOTAL DEPRESSION	0.19 IN	ACTIVE GRATE WEIR LENGTH	5.67 FT	TOTAL DEPRESSION	0.19 IN	ACTIVE GRATE WEIR LENGTH	5.67 FT	TOTAL DEPRESSION	0.19 IN	ACTIVE GRATE WEIR LENGTH	5.67 FT

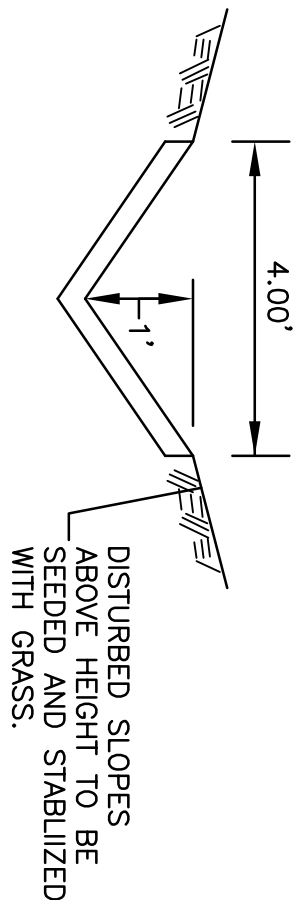
DITCH COMPUTATIONS

PROJECT DESCRIPTION: 15 YEAR STORM				PROJECT DESCRIPTION: 15 YEAR STORM				PROJECT DESCRIPTION: 15 YEAR STORM			
WORKSHEET TYPE: TRIANGULAR DITCH				WORKSHEET TYPE: TRIANGULAR DITCH				WORKSHEET TYPE: TRIANGULAR DITCH			
SOLVE FOR				SOLVE FOR				SOLVE FOR			
INPUT DATA:				INPUT DATA:				INPUT DATA:			
DISCHARGE	0.13 CFS	LEFT SIDE SLOPE	1.66 FT/FT	DISCHARGE	0.77 CFS	LEFT SIDE SLOPE	1.66 FT/FT	DISCHARGE	0.64 CFS	LEFT SIDE SLOPE	2.00 FT/FT
RIGHT SIDE SLOPE	1.66 FT/FT	ROUGHNESS COEFFICIENT	0.013	RIGHT SIDE SLOPE	1.66 FT/FT	ROUGHNESS COEFFICIENT	0.013	RIGHT SIDE SLOPE	2.00 FT/FT	ROUGHNESS COEFFICIENT	0.103
CHANNEL SLOPE	0.192 FT/FT	CHANNEL SLOPE	0.144 FT/FT	CHANNEL SLOPE	0.144 FT/FT	CHANNEL SLOPE	0.144 FT/FT	CHANNEL SLOPE	0.135 FT/FT	CHANNEL SLOPE	0.135 FT/FT
RESULTS:				RESULTS:				RESULTS:			
NORMAL DEPTH	0.11 FT	NORMAL DEPTH	0.23 FT	NORMAL DEPTH	0.23 FT	NORMAL DEPTH	0.23 FT	NORMAL DEPTH	0.27 FT	NORMAL DEPTH	4.43 FT/SEC
VELOCITY	6.51 FT/SEC	VELOCITY	9.12 FT/SEC	VELOCITY	9.12 FT/SEC	VELOCITY	9.12 FT/SEC	VELOCITY	4.43 FT/SEC	VELOCITY	1.21 FT/SEC

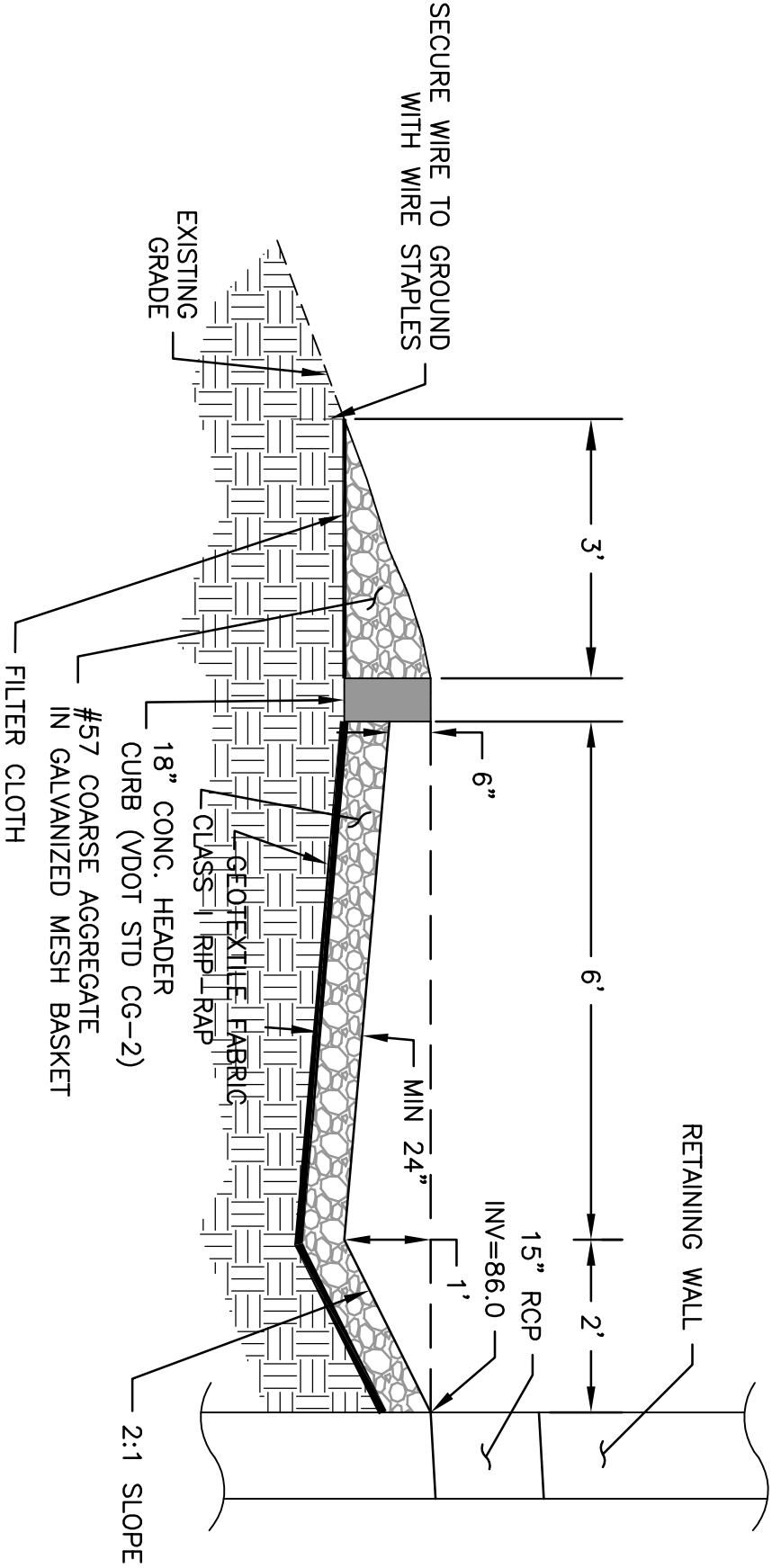
PROJECT DESCRIPTION: 15 YEAR STORM				PROJECT DESCRIPTION: 15 YEAR STORM				PROJECT DESCRIPTION: 15 YEAR STORM			
WORKSHEET TYPE: TRIANGULAR DITCH				WORKSHEET TYPE: TRIANGULAR DITCH				WORKSHEET TYPE: TRIANGULAR DITCH			
SOLVE FOR				SOLVE FOR				SOLVE FOR			
INPUT DATA:				INPUT DATA:				INPUT DATA:			
DISCHARGE	0.30 CFS	LEFT SIDE SLOPE	2.00 FT/FT	DISCHARGE	0.64 CFS	LEFT SIDE SLOPE	2.00 FT/FT	DISCHARGE	0.64 CFS	LEFT SIDE SLOPE	2.00 FT/FT
RIGHT SIDE SLOPE	2.00 FT/FT	ROUGHNESS COEFFICIENT	0.03	RIGHT SIDE SLOPE	2.00 FT/FT	ROUGHNESS COEFFICIENT	0.103	RIGHT SIDE SLOPE	2.00 FT/FT	ROUGHNESS COEFFICIENT	0.103
CHANNEL SLOPE	0.007 FT/FT	CHANNEL SLOPE	0.135 FT/FT	CHANNEL SLOPE	0.135 FT/FT	CHANNEL SLOPE	0.135 FT/FT	CHANNEL SLOPE	0.135 FT/FT	CHANNEL SLOPE	0.135 FT/FT
RESULTS:				RESULTS:				RESULTS:			
NORMAL DEPTH	0.36 FT	NORMAL DEPTH	0.27 FT	NORMAL DEPTH	0.27 FT	NORMAL DEPTH	0.27 FT	NORMAL DEPTH	0.27 FT	NORMAL DEPTH	4.43 FT/SEC
VELOCITY	1.21 FT/SEC	VELOCITY	4.43 FT/SEC	VELOCITY	4.43 FT/SEC	VELOCITY	4.43 FT/SEC	VELOCITY	4.43 FT/SEC	VELOCITY	1.21 FT/SEC



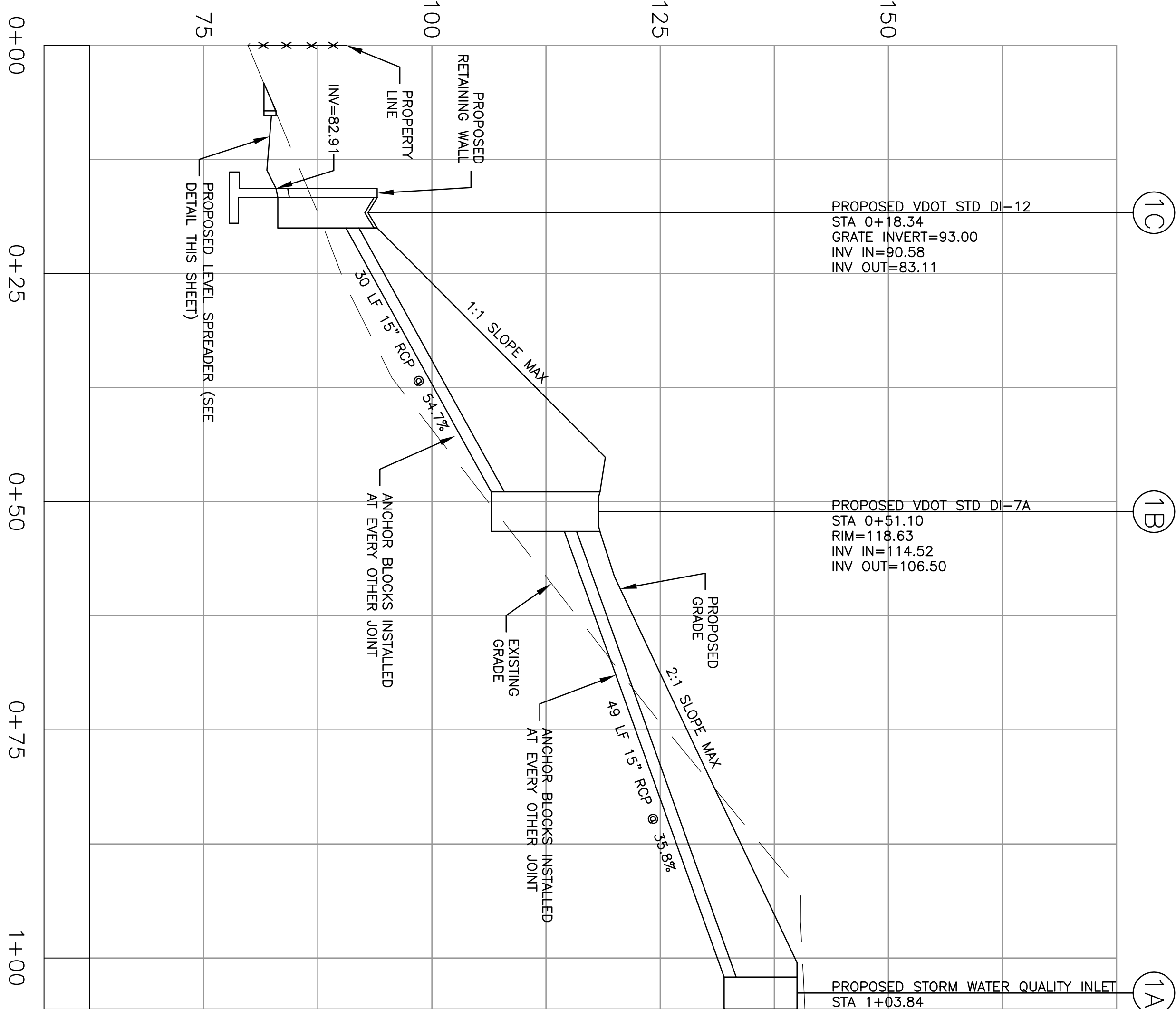
TYPICAL CONCRETE DITCH SECTION
NOT TO SCALE



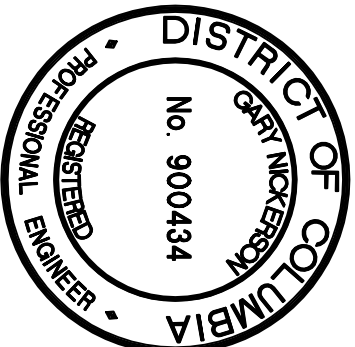
TYPICAL GRASS DITCH SECTION
NOT TO SCALE



LEVEL SPREADER WITH RIGID LIP
SCALE 1" = 2'



STORM SEWER
SCALE: 1" = 10'



KEY PLAN

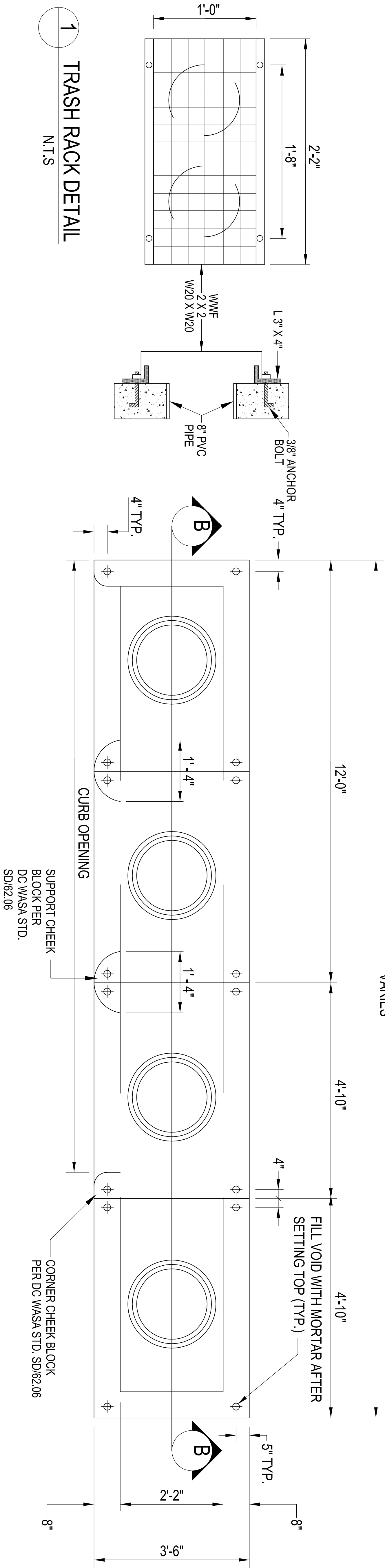
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No.	DATE	BY	Description

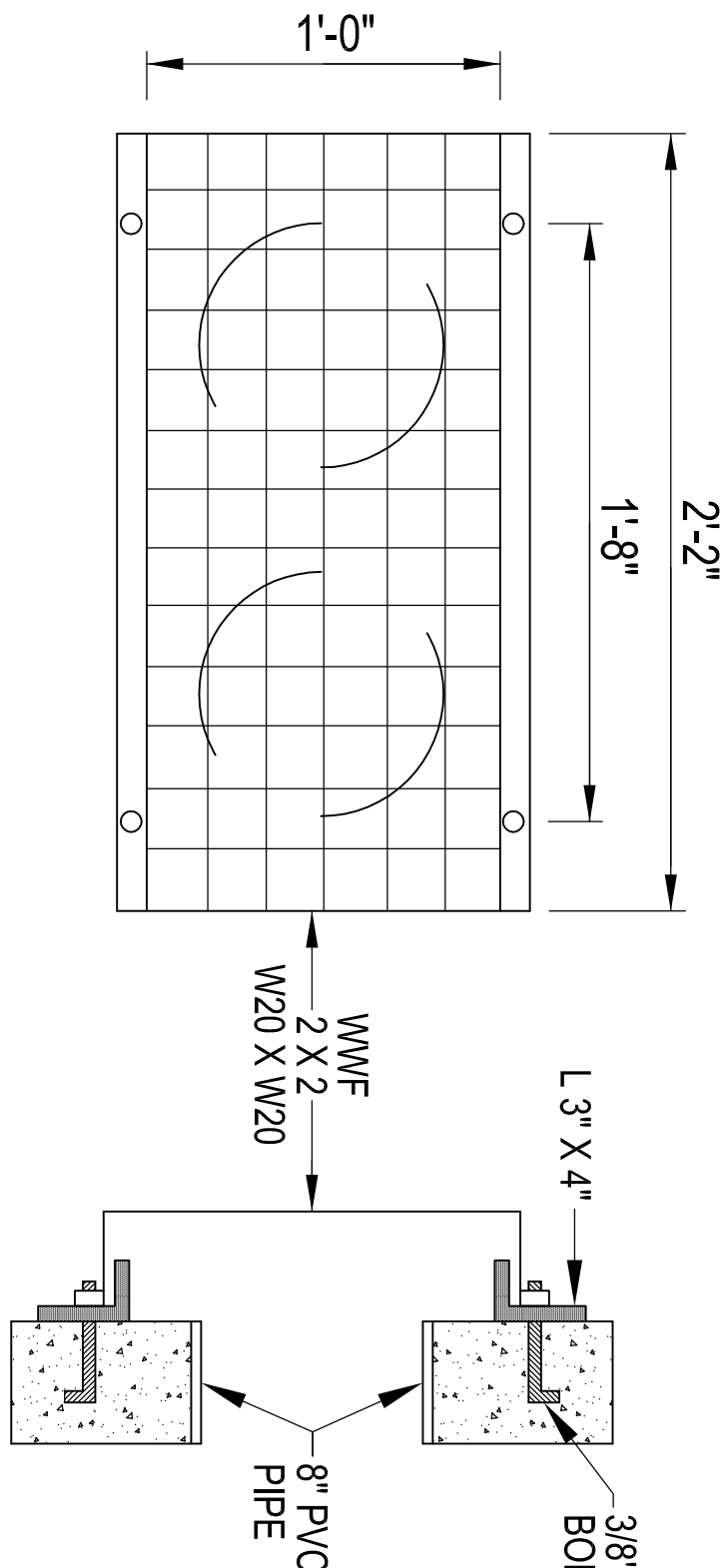
DRAWN BY	A. SPRUCH
APPROVED BY	T. CULLETON
CHECKED BY	T. CULLETON
DATE	JANUARY 2007

STORMWATER QUALITY INLET DETAILS

PROJECT NO. 05136000

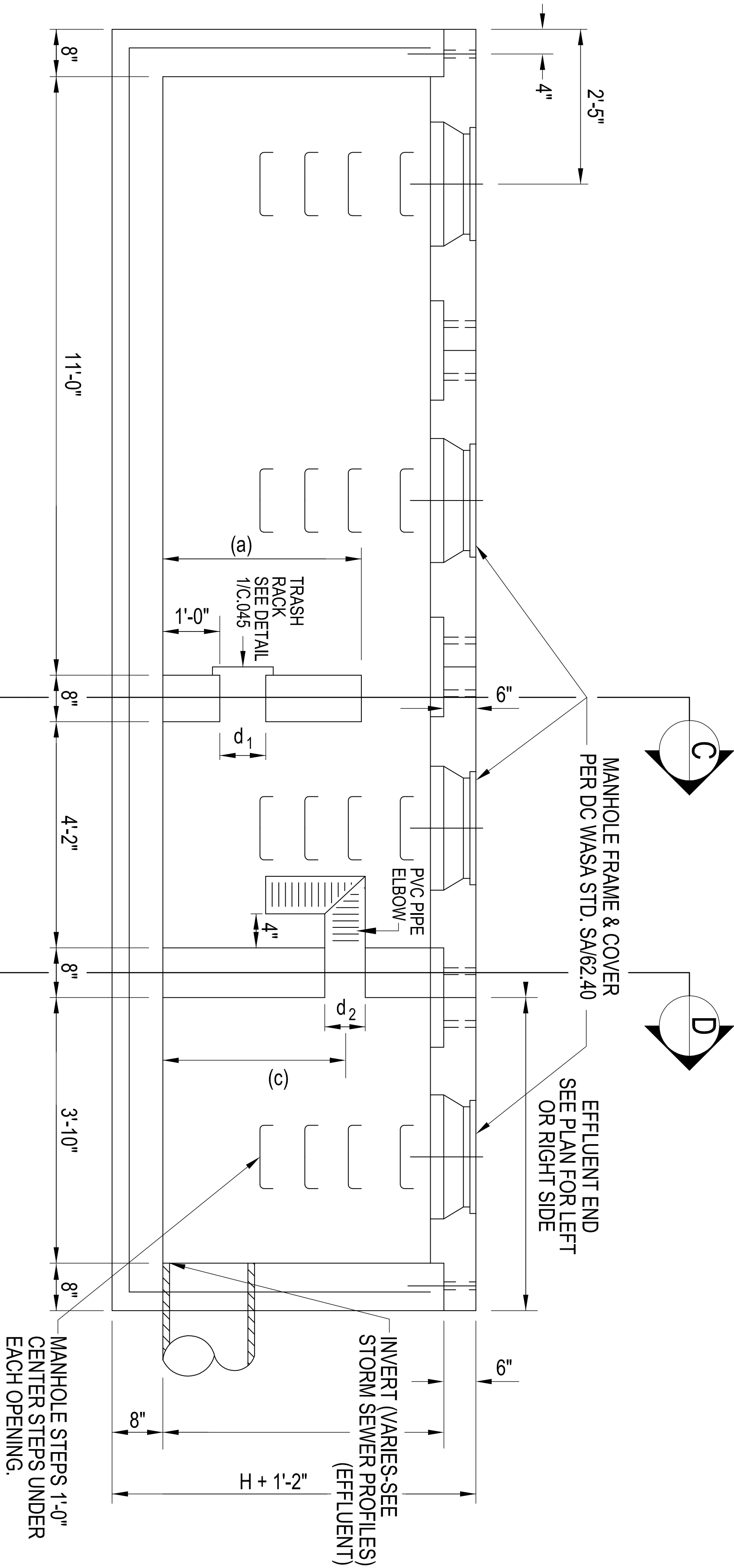


1 TRASH RACK DETAIL
N.T.S

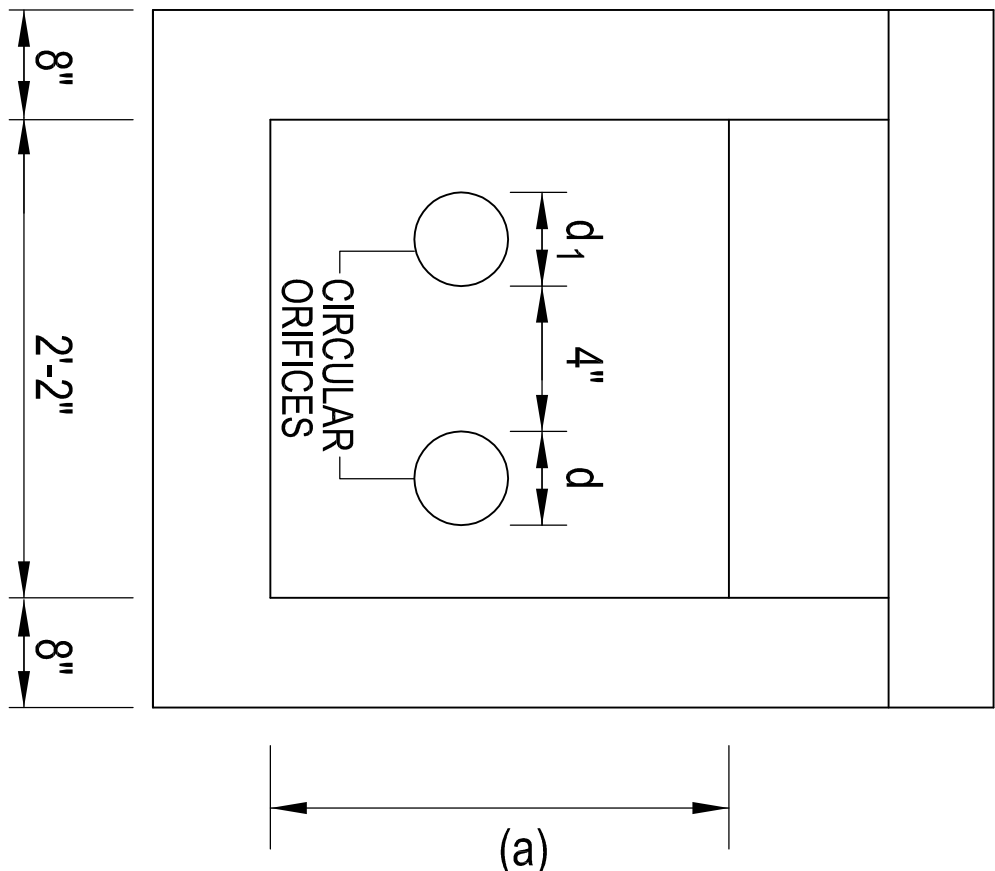


- NOTES
1. THE DESIGN AND FABRICATION OF ALL PRECAST MEMBERS SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE ACI 318.
 2. PRECAST STRUCTURES SHALL BE WATER TIGHT.
 3. PRECAST STRUCTURES SHALL BE DESIGNED FOR ALL APPLICABLE DEAD LOADS, INCLUDING EARTH AND FILL COVER INDICATED ON THE DRAWINGS.
 4. PRECAST STRUCTURES SHALL BE DESIGNED FOR AN H-20 STRUCTURAL LOAD RATING ACCORDING TO AASHTO HB 17.
 5. HEIGHT INDICATED IS MINIMUM. ALL UNITS SHALL BE SET SO THAT THEIR BOTTOM SURFACE IS LEVEL AND THE MINIMUM HEIGHT IS ACHIEVED ON THE DOWNGRADE SIDE OF THE UNIT. ADJUSTMENTS TO THE UNITS TOP TO MEET THE ROADWAY GUTTER GRADE SHALL BE ACCOMPISHED BY INSTALLING BRICKS WHERE NECESSARY, BETWEEN THE STRUCTURE AND ITS TOP.

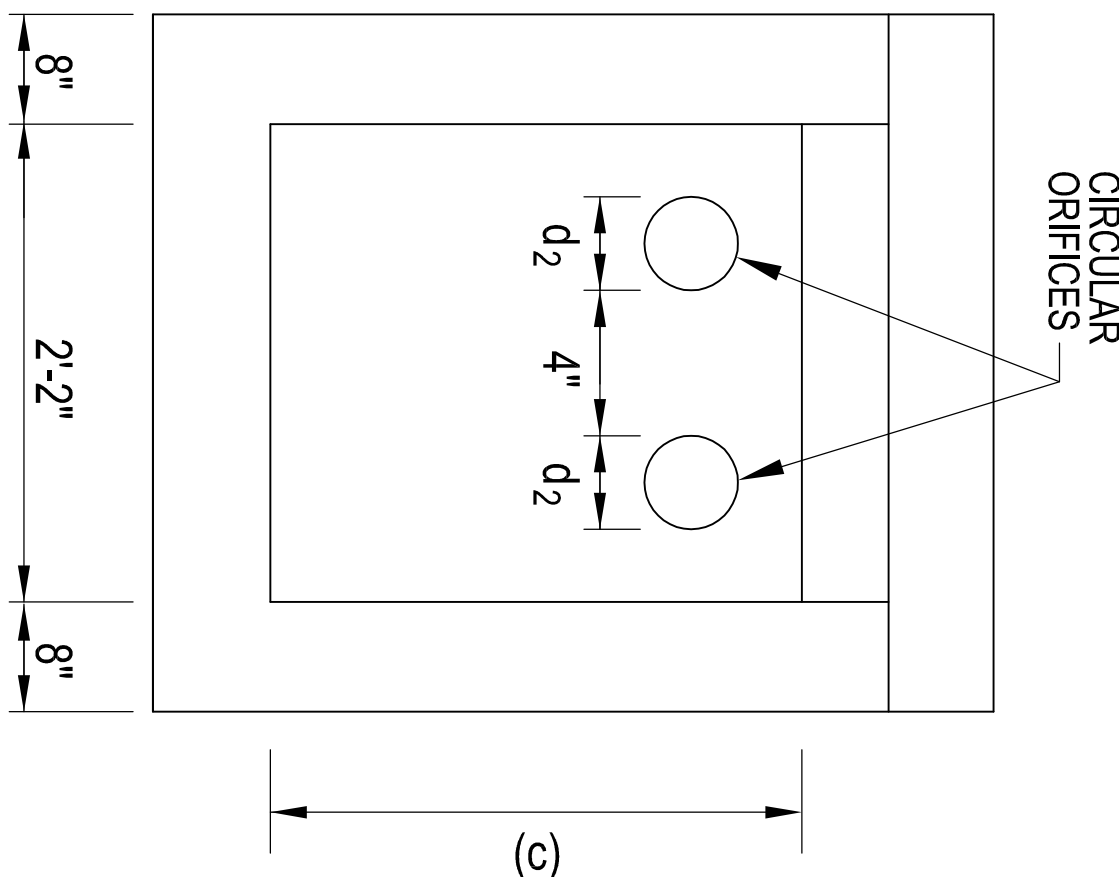
WATER QUALITY INLET PLAN DETAIL
N.T.S



C WATER QUALITY INLET SECTION DETAIL
N.T.S



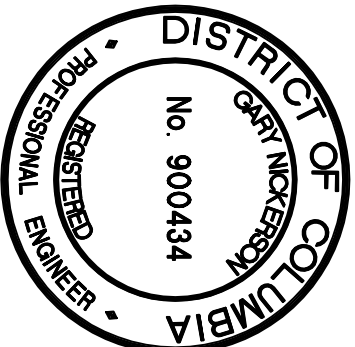
D WATER QUALITY INLET SECTION DETAIL
N.T.S



STRUCTURE	Q15(GFS)	Q2(GFS)	FIRST CHAMBER LENGTH (L)	HEIGHT (H)	(a)	(b)	(d1)	(d2)	PVC ELBOW Ø
1A	2.54	1.78	11'-0"	7'-6"	6'-4"	5'-6"	0'-8"	6'-11"	8"

B WATER QUALITY INLET SECTION DETAIL
N.T.S

WEST PERIMETER ROAD
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KEY PLAN

SCALE

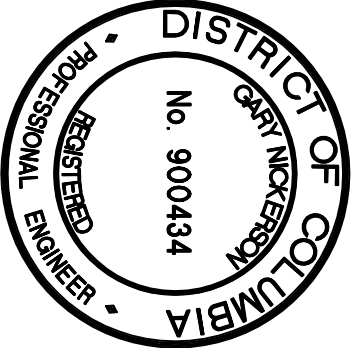


REVISIONS	
No.	Description

DRAWN BY A. SPRUCH
APPROVED BY T. CULLETON
CHECKED BY T. CULLETON
DATE JANUARY 2007

WEST PERIMETER ROAD
PROFILE

PROJECT NO. 05136000



KEY PLAN

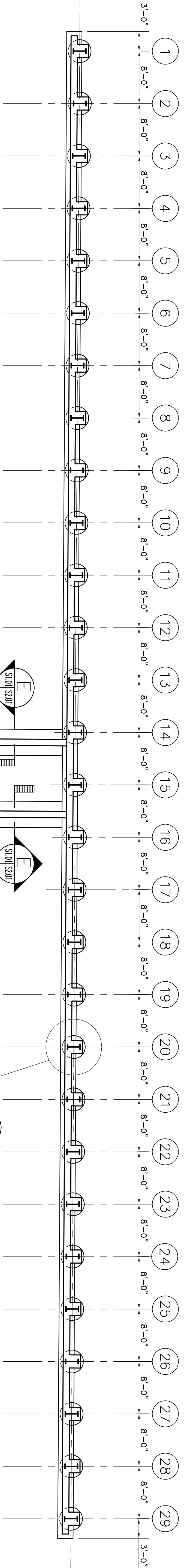
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No.	DATE	BY	Description

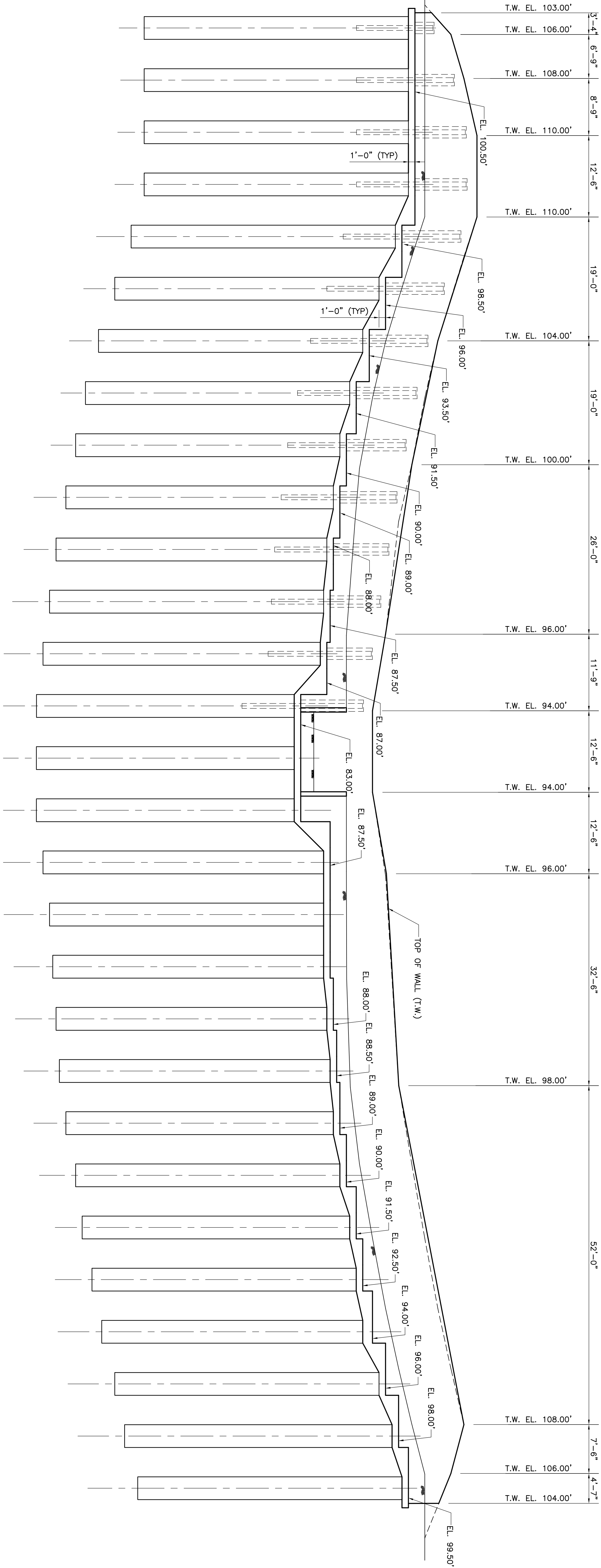
REVISIONS	No.	DATE	BY	Description

WALL PLAN
AND
ELEVATION

PROJECT NO. 05136000



PLAN
SCALE: 1/8"=1'-0"



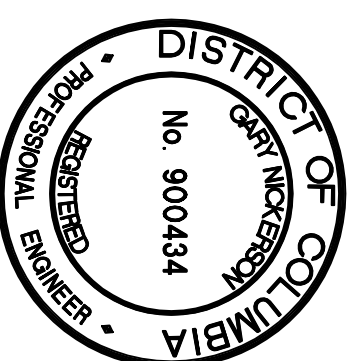
ELEVATION
SCALE: 1/8"=1'-0"



Dewberry & Davis LLC
8401 ARLINGTON BLVD
FAIRFAX, VA 22031
PHONE: 703.849.0100
FAX: 703.849.0118

WEST
PERIMETER
ROAD
GEORGETOWN
UNIVERSITY
WASHINGTON, DC

SEA




KEY PLAN

SCALE

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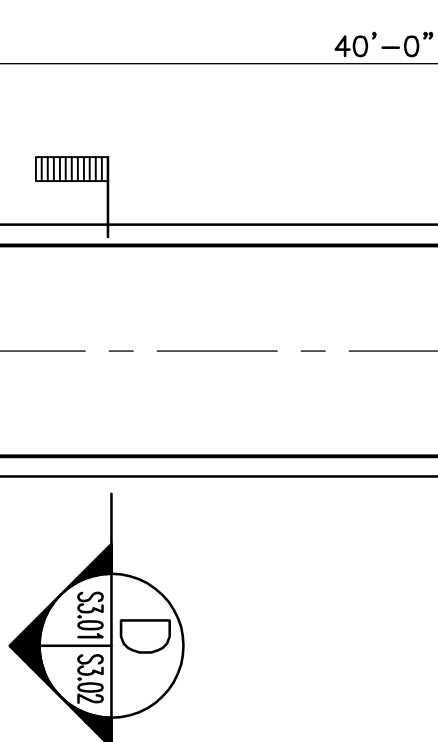
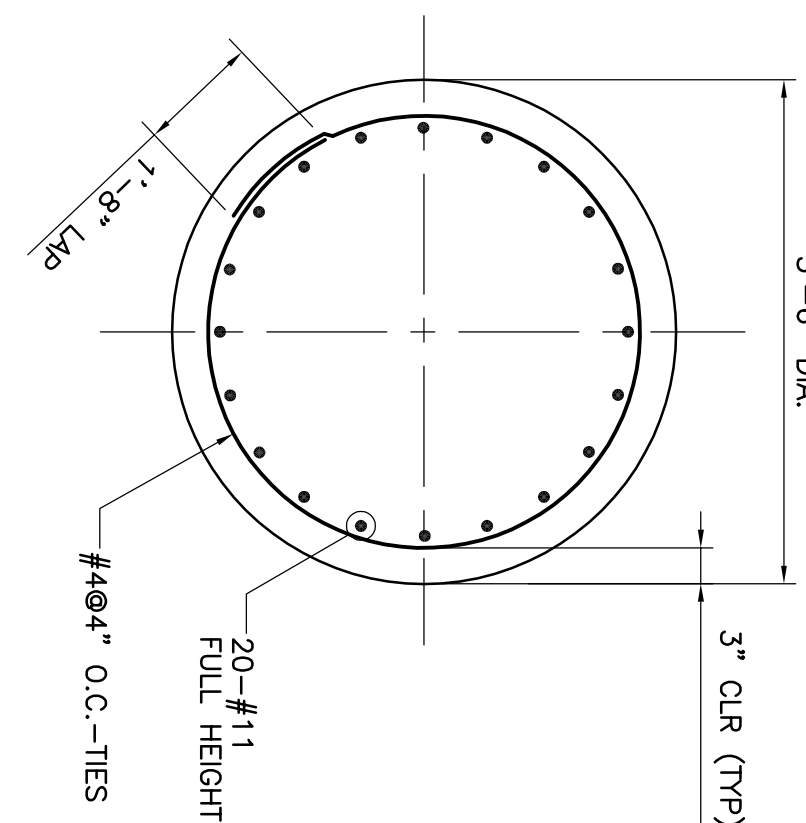
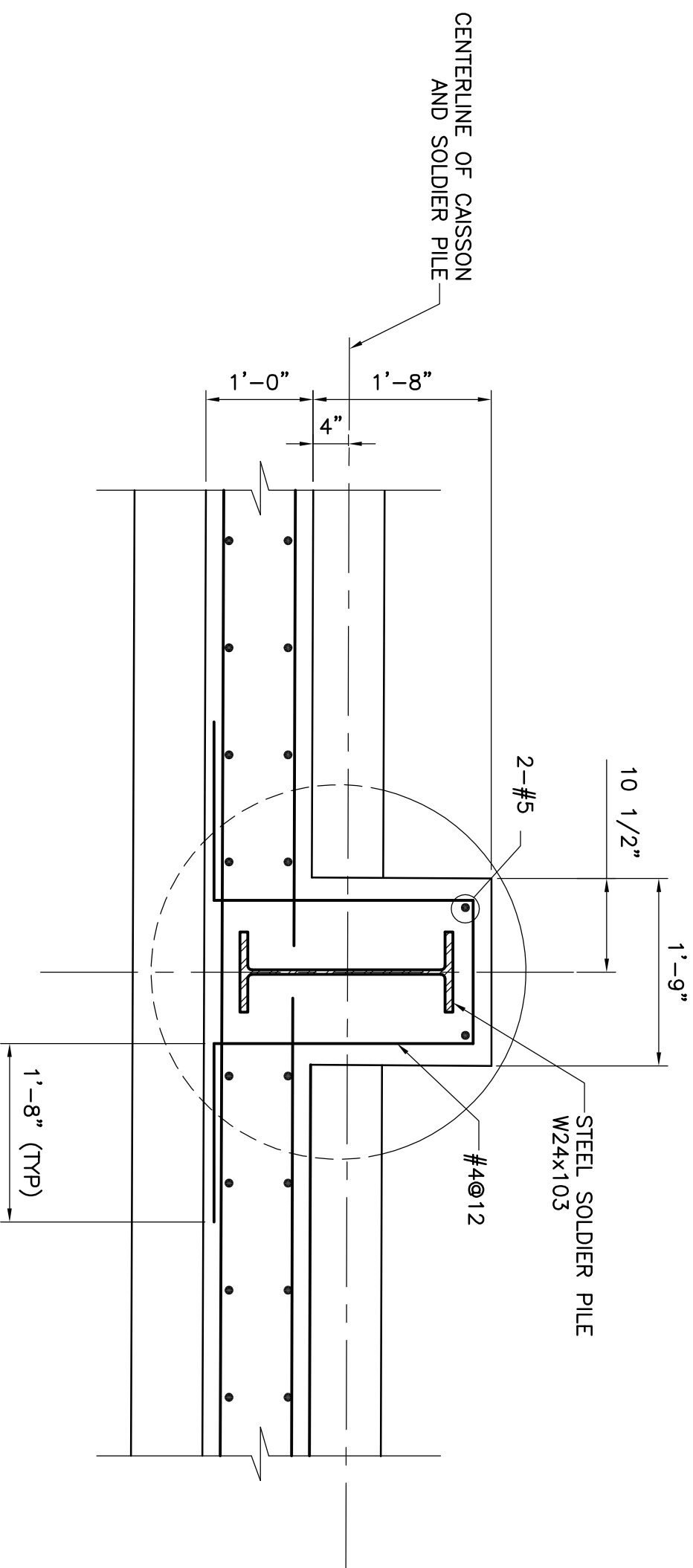
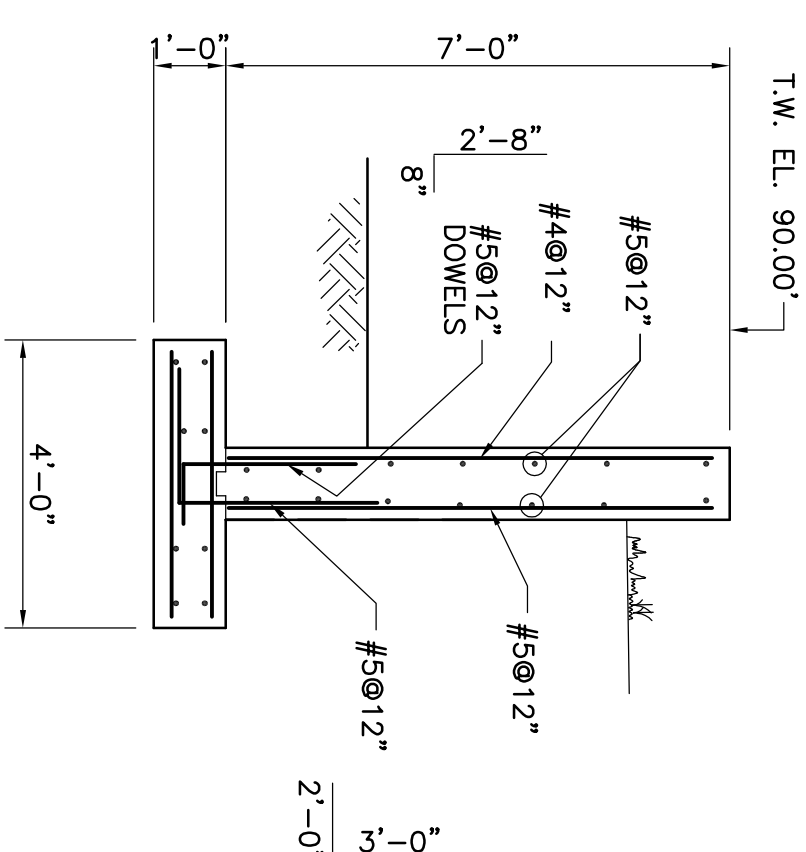
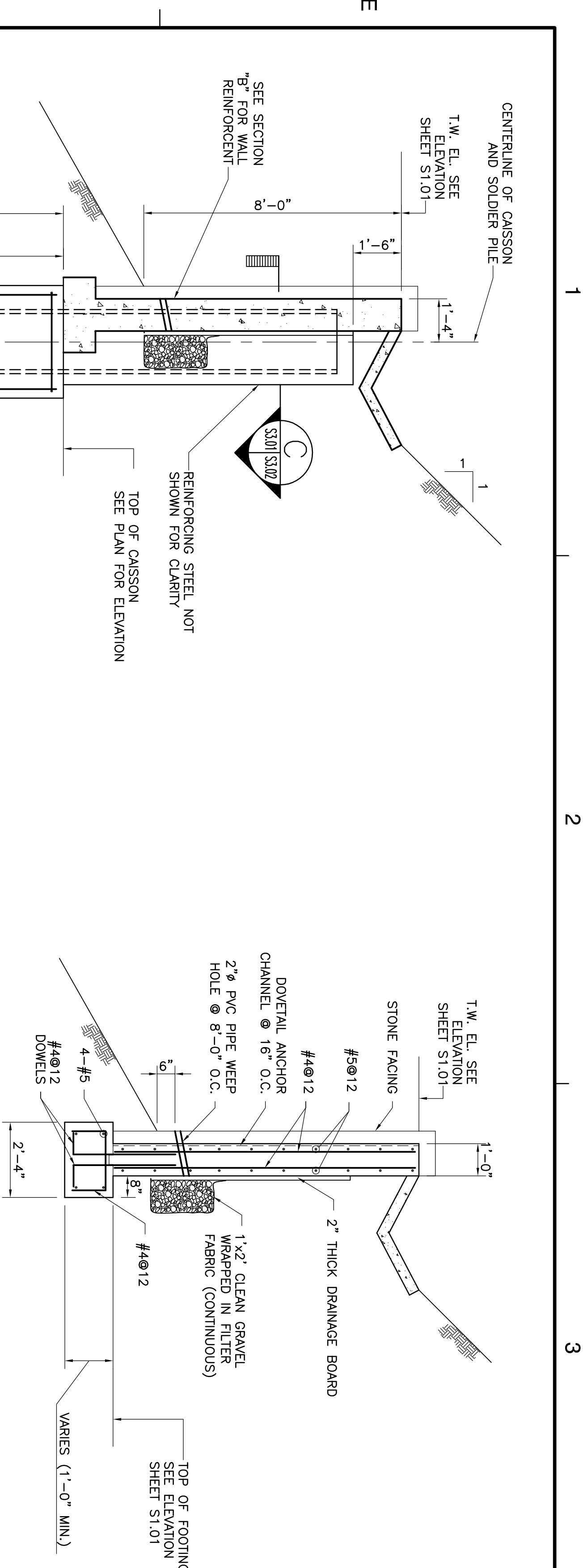
STRUCTURAL NOTES AND SECTIONS

PROJECT NO. 0513600C



SHEET NO

18 OF 18



STRUCTURAL NOTES:

A. GENERAL

1. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL DIMENSIONS INCIDENT TO THE WORK. ALL DIMENSIONS SHALL BE COORDINATED PRIOR TO PROCEEDING WITH LAYOUT, CONSTRUCTION OR FABRICATION. THE CONTRACTOR SHALL NOTIFY IMMEDIATELY THE ENGINEER OF ANY DIMENSIONAL DISCREPANCIES.
2. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK. THE ENGINEER WILL NOT ADVISE OR NOR ISSUE DIRECTIONS AS TO SAFETY PRECAUTIONS AND PROGRAMS.
4. THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING STRUCTURES, AND UTILITY CONDITIONS ADJACENT TO THE NEW WALL. IF CONDITIONS DIFFER FROM THOSE SHOWN, NOTIFY DEWEBERY IMMEDIATELY.

B. CODES:

1. INTERNATIONAL BUILDING CODES (I.B.C.) 2003.
2. BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE ACI 318 AND 301 CURRENT EDITION

C. CONCRETE

1. UNLESS NOTED OTHERWISE, ALL CAST IN PLACE CONCRETE SHALL BE AIR ENTRAINED WITH A MAXIMUM SLUMP OF 4 INCHES. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE ACI 318 AND ACI 301. CONCRETE SHALL ATTAIN MINIMUM 28 DAY STRENGTH OF 4000 PSI.

2. ALL REINFORCING STEEL FOR CONCRETE SHALL CONFORM TO ASTM SPECIFICATIONS TO A 615, GRADE 60 FOR BARS AND A 185 FOR WELDED WIRE FABRIC. REINFORCING DETAILS SHALL BE IN ACCORDANCE WITH CURRENT AO STANDARD PRACTICES FOR DETAILING REINFORCED CONCRETE STRUCTURES. BAR LAPS SHALL BE CLASS "B" TENSION LAPS. PROVIDE CONCRETE COVERS OVER REINFORCING BARS IN ACCORDANCE WITH AO 318. HOOKS SHOWN SHALL BE STANDARD HOOKS UNLESS OTHERWISE DIMENSIONED.

3. UNLESS SHOWN OTHERWISE, THE FOLLOWING CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT.

- b. CONCRETE EXPOSED TO EARTH OR WEATHER
- | | |
|-------------------------------|-----------|
| 1. NO. 6 THROUGH NO. 18 | 2 IN. |
| 2. NO. 5 AND SMALLER | 1 1/2 IN. |
| EXPOSED TO EARTH | 3 IN. |

D. FOUNDATION

1. THE CONTRACTOR SHALL REFER TO THE SOIL REPORT, PREPARED FOR THIS PROJECT FOR ALL REQUIREMENTS RELATIVE TO SITE PREPARATION, EARTHWORK, ETC..

2. SUITABLE BEARING CONDITIONS SHALL BE CONFIRMED BY A GEOTECHNICAL ENGINEER REGISTERED AS A PROFESSIONAL ENGINEER IN DISTRICT OF COLUMBIA AND REPORTED TO DENBERRY & DAVIS IN WRITING PRIOR TO CASTING FOUNDATIONS.

3. ALL SUBGRADE, FOOTING EXCAVATIONS, COMPACTED FILL AND BACKFILL SHALL BE INSPECTED AND TESTED BY A GEOTECHNICAL ENGINEER REGISTERED AS A PROFESSIONAL ENGINEER IN THE COMMONWEALTH OF VIRGINIA TO VERIFY CONFORMANCE WITH THE RECOMMENDATIONS HEREIN.